



## Geosynthetic Clay liners – Sustainable and resilient barrier applications in hydraulic engineering

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**Chairman:** CEN TC 189 WG6 Barriers and WG7;  
**Board:** The IGS Foundation, Technical Advisory Committee Geosynthetics  
**Lecturer:** University of Applied Sciences Bielefeld, University of Applied Sciences Ostwestfalen-Lippe

### Content of the presentation

## Topics

- CEN TC189/WG7
- ISO/TR 18228-9 Design using geosynthetics — Part 9: Barriers
- Water stress and rainfall
- Failure in past designs
- Dykes/levees
- Underwater installation canals
- Hydropower dams
- GCLs - sustainable, ecological, economical, resilient
- Summary

Content of the presentation

CEN TC189 WG7 proposed scope

Requirements related to the revised Construction Products Regulation (CPR) common to Geosynthetics, including sustainability and environmental topics, such as:

- Release of dangerous substances;
- Environmental performance;
- Circular Economy;
- Potential release of microplastics during the different stages of the life cycle of the product.

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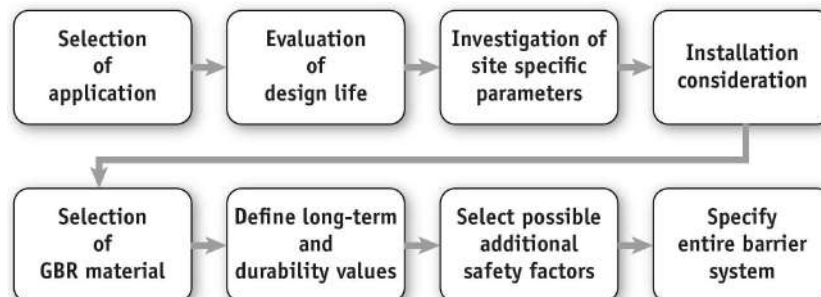
Geosynthetic barrier design guide

ISO/TR 18228-9 Design using geosynthetics - Barriers

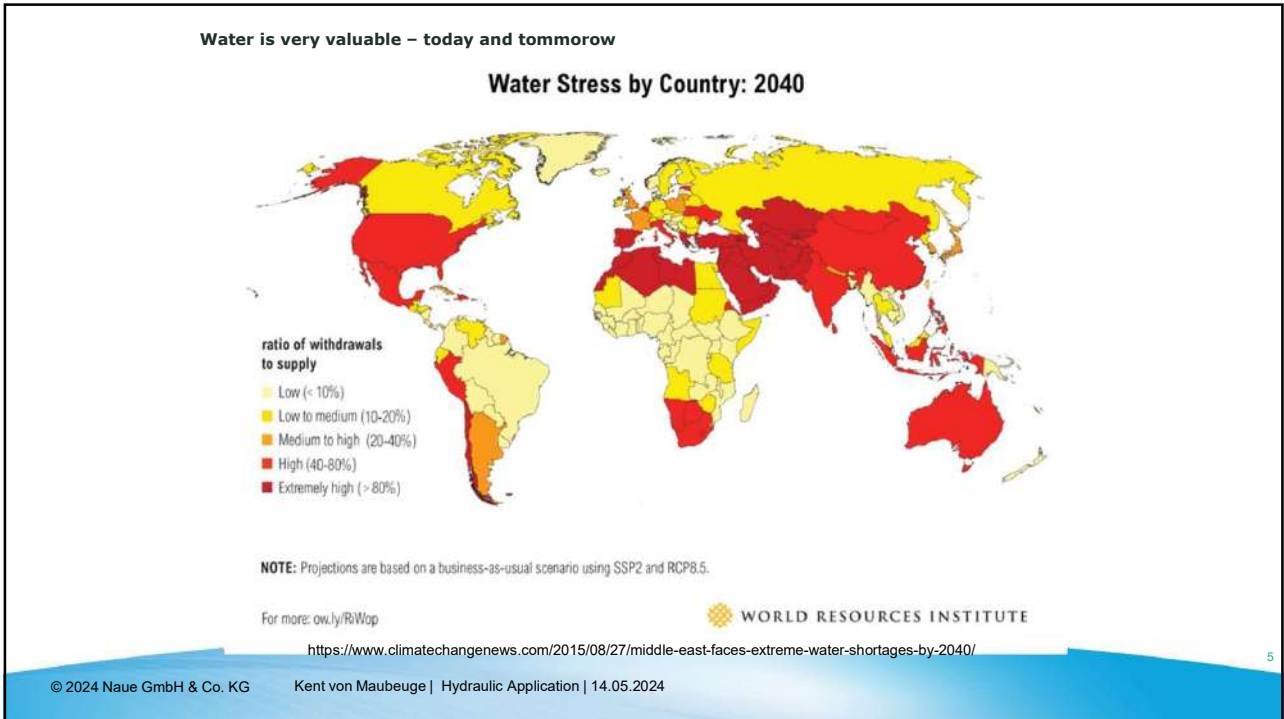
**The reason** Introduction to geosynthetic newcomers.

**The Result:** Show new technologies rather than using conservatism and past approaches.

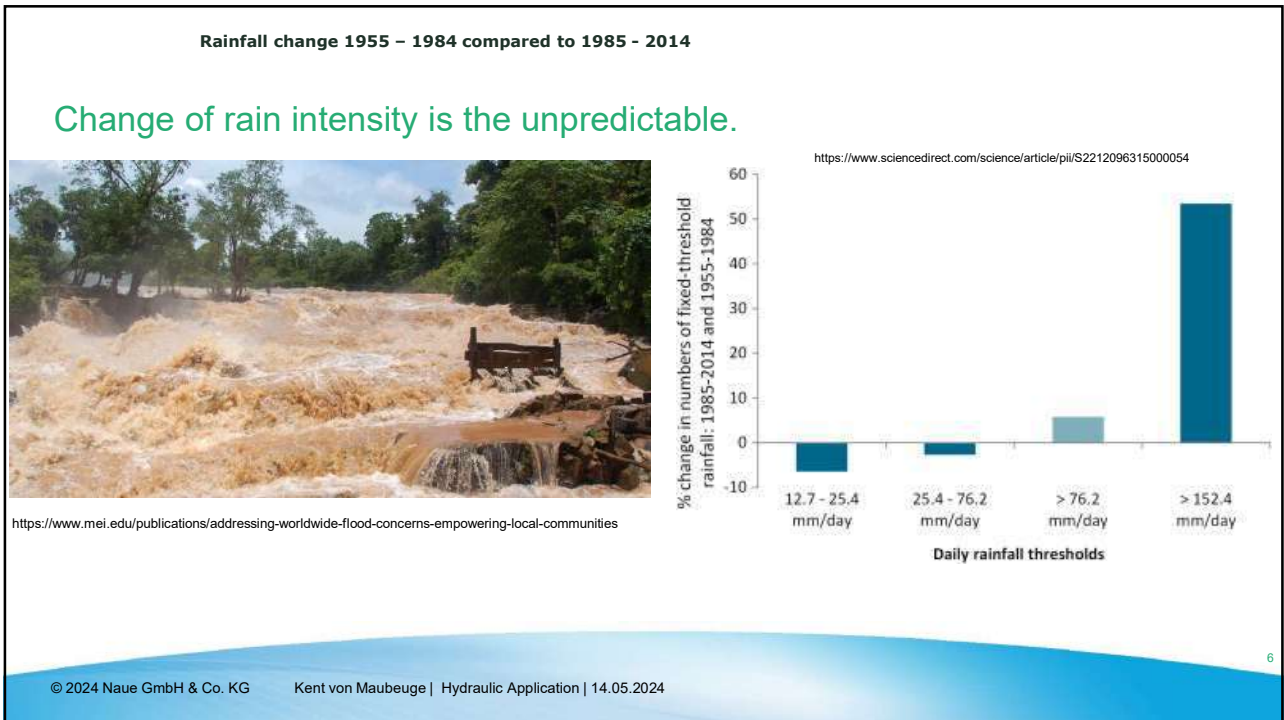
**The Solution:** Educating and removing the fears or concerns about geosynthetic solutions



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Unsecure levees

### Flood Event in Germany

More than 100 dykes bursted and caused floods around River Mulde Aug. 2002



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Life-saving levees

### Goondiwindi's levee, Australia, saves the town from flooding, but surrounding areas are under water (2022)

As flood risks increase, it's time to recognize the importance but also the limits of levees



<https://www.abc.net.au/news/2021-12-04/qld-goondiwindi-levee-saves-town-from-flooding/100674936/>

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Unsecure levees

### Flood Event in Germany

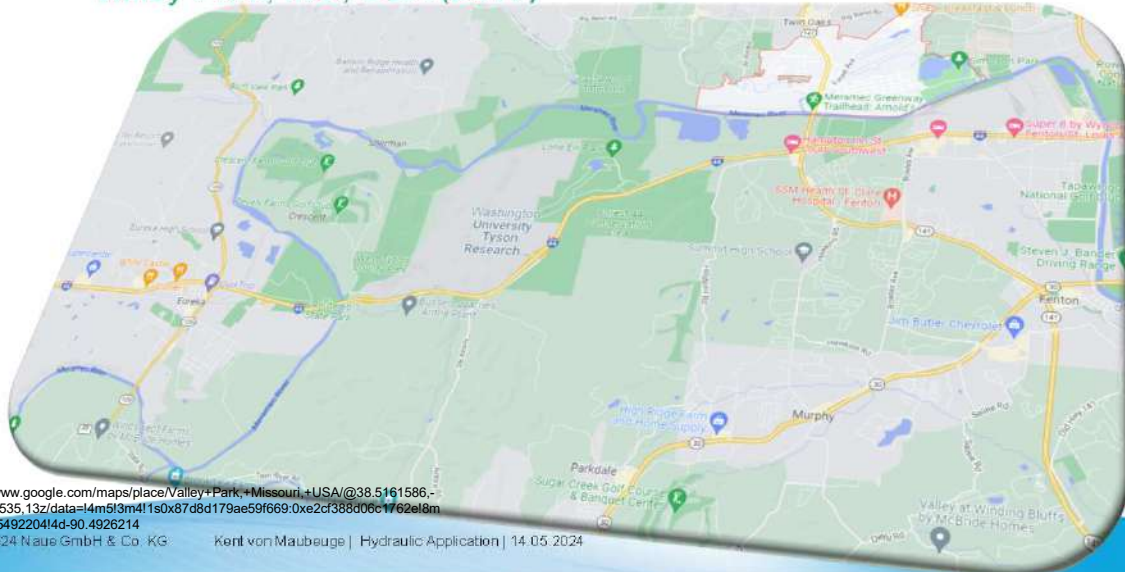
Main causes of the levees bursting



- Many levees > 50 years old
- Animals forming burrows
- Longer, drier summers
- Cracking of levee
- Root formation
- Designed to withstand water pressure for a shorter period
- Heavier rainfalls
- Longer exposure to water pressure
- Built too close to river

Secure new and higher levees

### Meramec River flood event in in Valley Park, MO, USA (2017)



<https://www.google.com/maps/place/Valley+Park,+Missouri,+USA/@38.5161586,-90.5769535,13z/data=!4m5!3m4!1s0x87d8d179ae59f669:0xe2cf388d06c1762e18m2!3d38.549220414d-90.4926214>

Secure new and higher levees

Meramec River flood event in in Valley Park, MO, USA (2017)



<https://youtu.be/LTv6RkFneIM>

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No or too low levees and more flood water due to higher levees upstream

Meramec River flood event in Eureka, MO, USA and the consequences (2017)

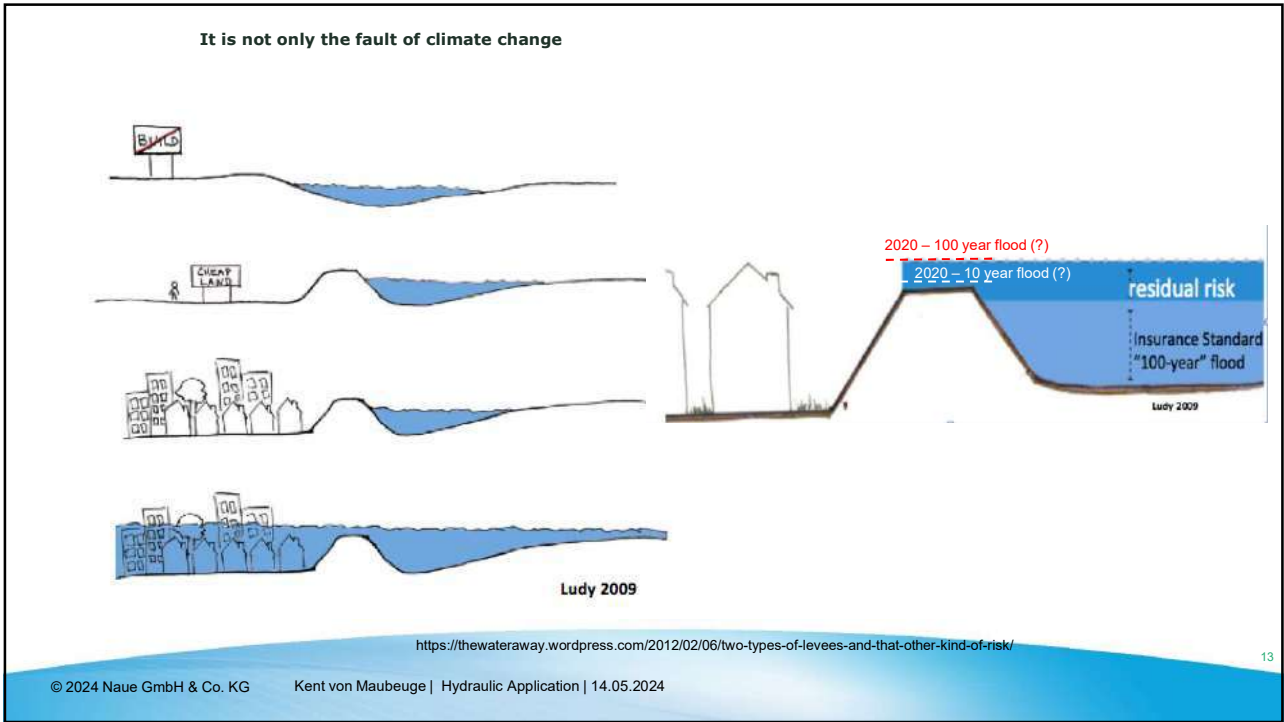


<https://youtu.be/LTv6RkFneIM>

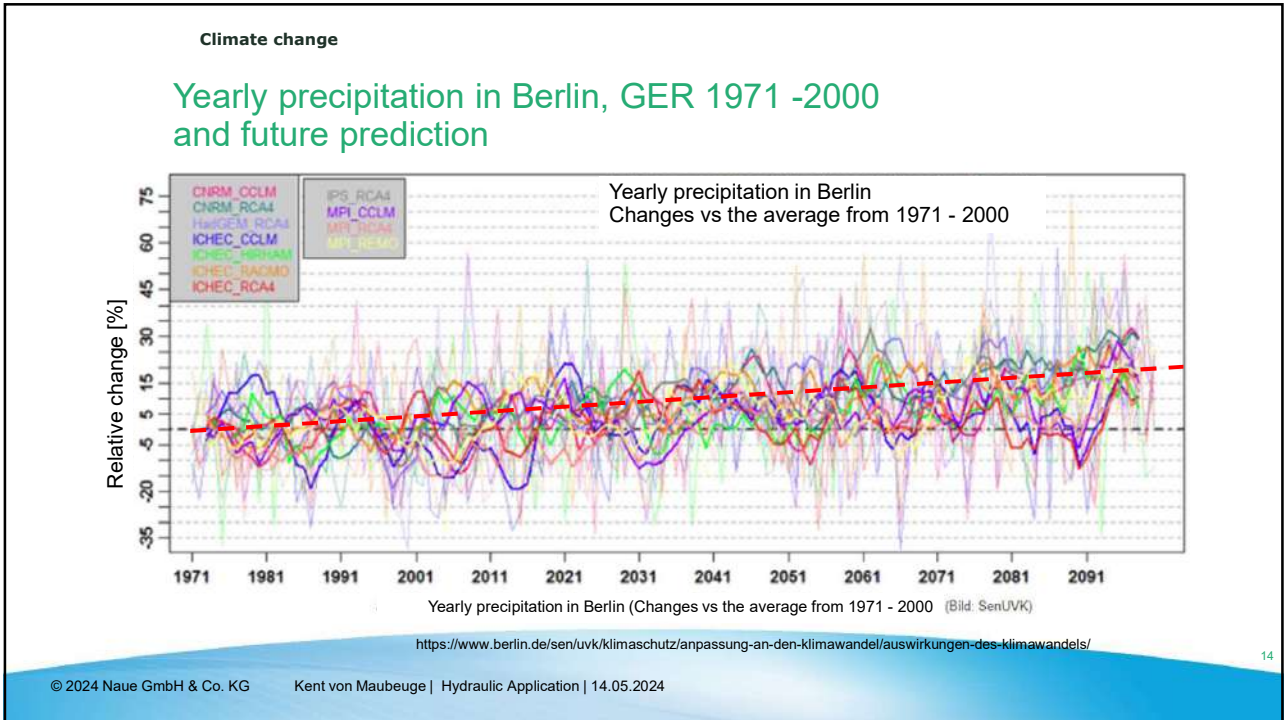
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### Damage to levees

#### Mechanisms of damage to levee

**Overtopping**  
If levees are not high enough

**Erosion**  
If flow is higher and lasts longer

**Seepage failure**  
If flood lasts longer and permeation is high

<https://www.pwri.go.jp/eng/about/pr/webmag/wm048/kenkyu.html>

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### Fundamentals in designing river levees

## The 3 Zone Levee Concept

(DVWK-Guideline „River dykes“ 210/1986 or DIN 19712)

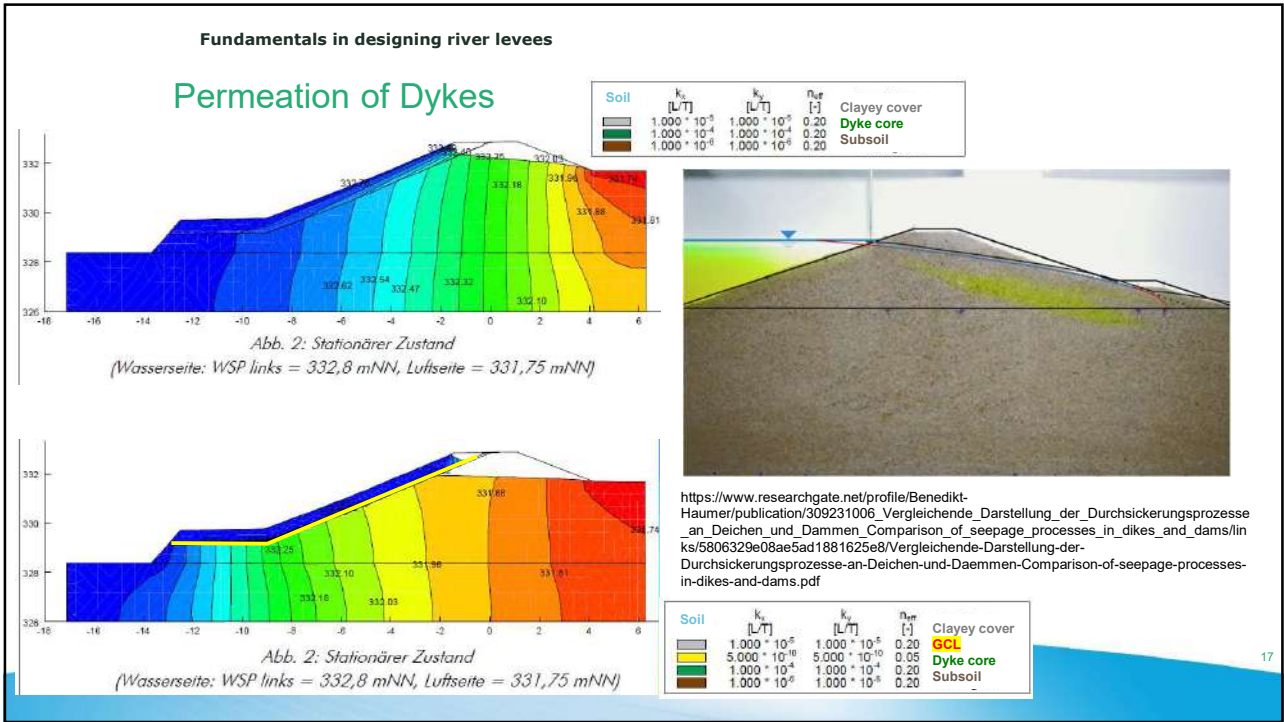
**1 impervious**  
**2 Permeable supporting**  
**3 Very permeable**

Erosion control  
Estimated 100 year flood level  
Upstream  
Downstream  
Dyke defence road  
Geogrid embankment reinforcement  
Nonwoven geotextile filter  
Clay or GCL

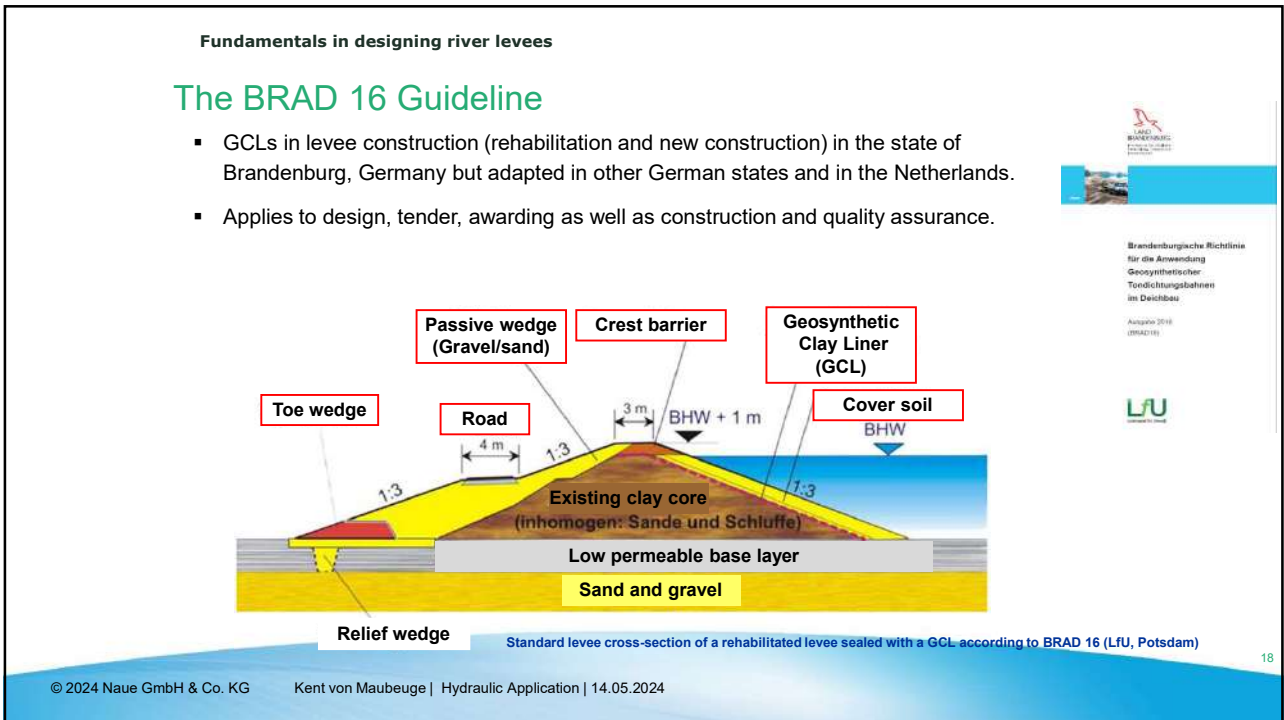
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Requirements for levee classes

The BRAD 16

Typen: A (needle-punched)



B (needle-punched)



C (stitch-bonded)



Anlage 1: Anforderungskatalog

Zeile	Kennwert der GTD	Norm	Deichklasse <sup>1)</sup>	
			II und III	≥ 3m
1	Flächenbezogene Masse des Tonbestandteils bei ≤ 15 % Wasseranteil Montionitgehalt	DIN EN 14195 VDG P 69 [1]	natürliches oder aktiviertes Natrumterfontit ≥ 4.500 g/m <sup>2</sup> als Pulver oder Granulat Methylenblauwert ≥ 900 mg/g <sup>2)</sup>	
2	Durchlässigkeitsbeiwert K <sub>0,2</sub> <sup>3)</sup>	DIN EN 16416	< 0,1 · 10 <sup>-10</sup> m/s	< 1 · 10 <sup>-10</sup> m/s
3	Deck- und Trägergeotextilien <sup>4)</sup>	DIN EN ISO 9864 DIN EN ISO 10016	Geogewebe (GTX-W) ≥ 180 g/m <sup>2</sup> Geovliesstoff <sup>5)</sup> (GTX-O) ≥ 250 g/m <sup>2</sup> Geovliesstoff (GTX-N) ≥ 150 g/m <sup>2</sup>	Geogewebe (GTX-W) ≥ 200 g/m <sup>2</sup> Geovliesstoff <sup>5)</sup> (GTX-O) ≥ 350 g/m <sup>2</sup> Geovliesstoff (GTX-N) ≥ 300 g/m <sup>2</sup>
4	Durchdrickwiderstand <sup>6)</sup>	DIN EN 12236	≥ 1,5 kN	≥ 3,0 kN
5	Höchstzugkraft bei Geovliesstoffen in MC/CMD <sup>7)</sup> 1)	DIN EN ISO 10319	≥ 12 kN/m	≥ 15 kN/m
6	Höchstzugkraft bei Geogeweben und Mischprodukten in MC/CMD <sup>7)</sup> 2)	DIN EN ISO 10319	≥ 35 kN/m	≥ 50 kN/m
7	Dehnung bei Bruch der GTD in MC/CMD	DIN EN ISO 10319	≥ 10 %	≥ 30 %
8	Verbindfestigkeit im Scherversuch (MD)	DIN EN ISO 13426-2 <sup>8)</sup> ASTM D 5496 <sup>9)</sup>	≥ 60N/100m	≥ 100N/100m
9	Langzeitscherfestigkeit <sup>10)</sup> 1)	LAGA [8]	Böschungseigung 1:3 ≥ 15 kN/m <sup>2</sup>	
10	Langzeitscherfestigkeit <sup>10)</sup> 2)	LAGA [8]	Böschungseigung 1:2,5 ≥ 18 kN/m <sup>2</sup>	

11	äußere Standsicherheit	DIN EN 1054 DIN 4084	Nachweis der Sicherheit gegen Abgleiten in den Scherzust. GTD- Böden (für Träger- und Deckgeotextil) mit den lokalen bodenmechanischen Reibungsparametern
12	Erosionsbeständigkeit <sup>11)</sup>	RPW [3]	Benennungswerte ≤ 5% (nach DIN EN 13361:2013, B.4-4) bei einer Versuchsdruck von 95 98k
13	Nachweis der Langzeitbeständigkeit für GTD	DIN EN 13361, Anhang Dauerhaftigkeit, aktuelle Ausgabe	≥ 25 Jahre in natürlichen Böden mit einem pH-Wert von 4 bis 9 und einer Bodenlempertemperatur ≤ 29° Leistungsanforderung DoP nach CPR
14	Nachweis der Langzeitbeständigkeit für Deck- und Trägergeotextilien	DIN EN 13258: 2016-12, Prüfung 5, Dauerhaftigkeit	≥ 100 Jahre in natürlichen Böden mit einem pH-Wert von 4 bis 9 und einer Bodenlempertemperatur ≤ 29° Als Zusatzanforderung auf der Leistungsanforderung DoP oder als gesonderte Deklaration des Herstellers
15	Umweltverträglichkeit der GTD	MSGeE 2016 [1] Kapitel 6.29 und 7.7	Nachweis der Unbedenklichkeit

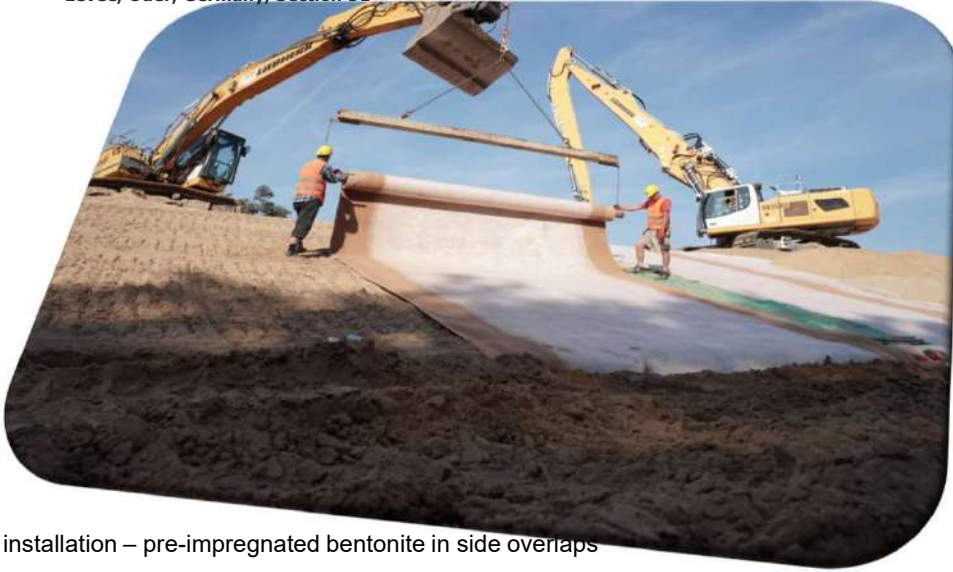
1) Deichklasse nach DIN 10712, Tab. 1, Bauwerksklasse - 3 m über landseitigen Gelände  
 2) Deichklassen nach DIN 10712, Tab. 1, Bauwerksklasse - 3 m über landseitigen Gelände. Diese Spalte gilt auch für Dache der Klasse II und III mit zu erwartenden Belastungen = 0,5 m  
 3) bei 30 kPa Nachweis bei 100 cm und 20°C. Die Durchlässigkeit ist auch für den Übergangsbereich nachzuweisen  
 4) nachzuweisen an den Ausgangsprodukten, die im Endzustand eine Trennung der Komponenten nicht möglich ist.  
 5) gilt für mehrlagige Vliesstoff-Gewebe-Kombinationen und für Geovliesstoffe mit stabilisierter Gewebestruktur. Das Mindestgewicht für die jeweilige Komponente beträgt 100 g/m<sup>2</sup>.  
 6) nachzuweisen an Endprodukt.  
 7) gilt auch, wenn eine stabilisierende Gewebestruktur (Vliesstoff vorhanden) mit oder ohne Geotextil- und Vliesstoff (nicht miteinander verbunden) sind.  
 8) Mitgeliefert auch Werte vor einem der Träger- oder Deckgeotextilien aus Geogewebe besteht.  
 9) Die Anforderungen an die Zugfestigkeit müssen auch von allen Einzelteilen der GTD erfüllt werden, d.h. die erste Peak der Kraft-Dehnungsdiagramme und damit das Verhalten der ersten Geotextilkomponente ist das bestimmende Kriterium für die Zugfestigkeit.  
 10) Analyse & CO<sub>2</sub> Komponenten für GTD. Experten eines Langzeitversuchs mit ≥ 10.000 h, nachweis gemäß Bundesminister für Umwelt, Naturschutz und nukleare Sicherheit (BMU) 5-5 Oberflächenabflusskonventionen aus gesamtstaatlichen Testanforderungen vom 02.12.2016 - Abschn. 5.7 der LAGA  
 11) Mühen bei neu entwickelten Produkten nach keine Untersuchungen der Langzeitbeständigkeit vorliegen, wird für einen Übergangszeitraum bis Ende 2017 der Nachweis der Verbundbeständigkeit vor = 100kN/m<sup>2</sup> im Norm-Zustand akzeptiert.  
 12) Das Produkt muss abweichend von der RPW beauftragt. Nach RPW wird nach dem Turnuswechsel die Durchlässigkeit bestimmt.  
 13) bevorzugt für verarbeitete Produkte  
 14) bevorzugt für verarbeitete Produkte  
 15) entspricht ca. 70 % Marktanteil

Levee, Oder, Germany, Section 51



Bird's view

Levee, Oder, Germany, Section 51



GCL installation – pre-impregnated bentonite in side overlaps

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Levee, Oder, Germany, Section 51



GCL anchor trench

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Reason for relying on a GCL (in GCL they trust)

### GCL advantages



- Thinner equivalent sealing system
- Less excavation on site required
- Faster to install than clay
- Manufacturing quality controlled
- Higher resistance to settlement, desiccation, frost/thaw cycles
- High resilience and erosion stable
- Slope stability
- Long-term durable (confirmed by excavations)
- Cost effective

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Levee in the Netherlands

### following basics of BRAD 16



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GCL underwater installation

How can a GCL be installed underwater?

- Installation of canal liners – **without lowering existing water levels**
- Enormous challenging - **Low to no visibility, uplift-forces, flow velocities**



GCL requirements:

- Low hydraulic conductivity
- Easiness of installation and QA
- Safe and reliable (in the area and for connections, overlaps)
- Robustness
- Economic and environmental friendly

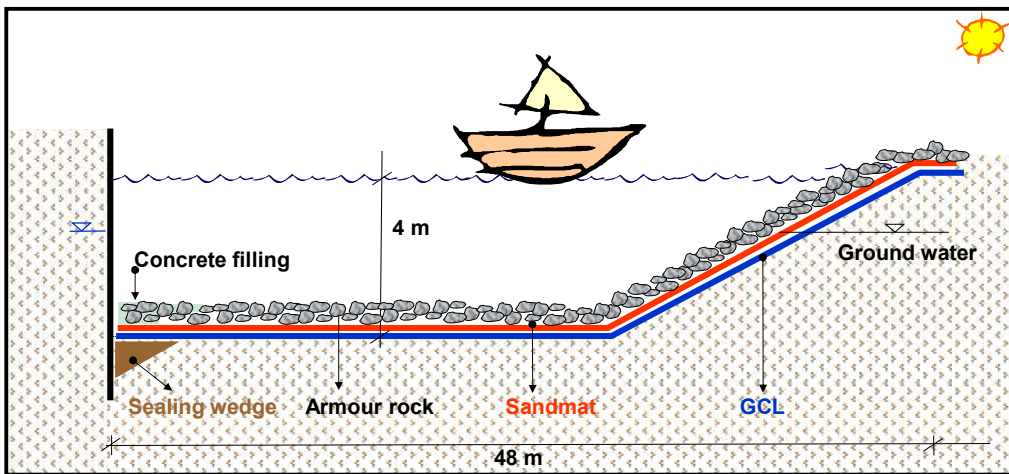
Empfehlungen zur Anwendung von Oberflächendichtungen an Seile und Böschung von Wasserstraßen

Abgeändert nach: DWA 1080  
 Herausgeber: BGR 180, 1999 (DIN 1080)  
 Bearbeiter: BGR 180, 1999 (DIN 1080)  
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 Bearbeiter: BGR 180, 1999 (DIN 1080)  
 Herausgeber: BGR 180, 1999 (DIN 1080)  
 Bearbeiter: BGR 180, 1999 (DIN 1080)

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GCL underwater installation

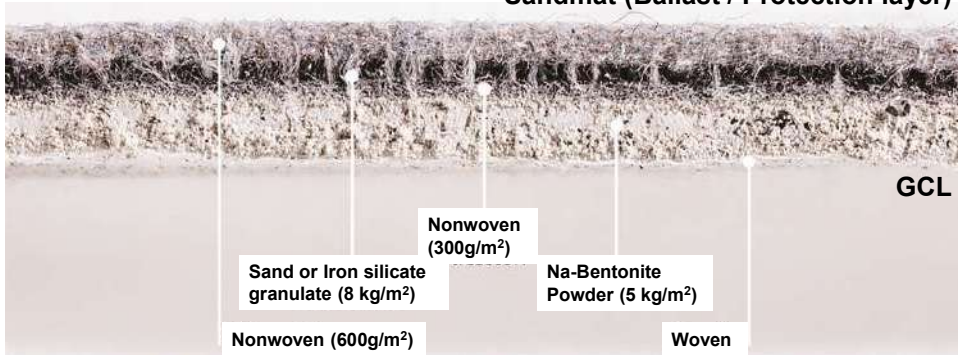
Specific requirements (Overlaps, connections, protection)



GCL underwater installation

GCL specific requirements

Sandmat (Ballast / Protection layer)



- The sand layer acts as a ballast for sinking,
- is a confining pressure against swelling of the bentonite and
- acts as a protection layer against dropping armour rock

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The composite image shows the installation of a GCL in a river. On the left, a crane is lifting a roll of GCL. In the center, a crane is installing a roll of GCL on the riverbed. On the right, a cross-section diagram shows the GCL being installed on a riverbed. The diagram is divided into two parts: 1. The GCL is being installed on a riverbed. 2. The GCL is being installed on a riverbed. The diagram shows a cross-section of the riverbed with a concrete wall on the left and a river on the right. A crane is lifting a roll of GCL from the riverbed. The GCL is being installed on the riverbed. The diagram is divided into two parts: 1. The GCL is being installed on a riverbed. 2. The GCL is being installed on a riverbed. The diagram shows a cross-section of the riverbed with a concrete wall on the left and a river on the right. A crane is lifting a roll of GCL from the riverbed. The GCL is being installed on the riverbed. The diagram is divided into two parts: 1. The GCL is being installed on a riverbed. 2. The GCL is being installed on a riverbed.

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GCL underwater installation

Danube river concrete rehabilitation, Ellgau, Bavaria



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Huge stormwater retention dam

Repair of a clay liner sealed dam, Glashütte, Germany

Project

- Old dam destroyed during flood event in 2002 (overflowing)
- First immediate repair with a smaller dam, to be increased in height later
- Height: 28 m, Length: ca. 180 m,
- Slopes: upstream (V:H) 1:2,5  
downstream (V:H) 1:2 to 1:2,8
- Lower dam approx. 16 m high with clay core  
Top dam approx. 12 m with GCL
- Water storage capacity >1 mio m<sup>3</sup>



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Huge stormwater retention dam

Repair of a clay liner sealed dam, Glashütte, Germany

GCL Requirements



Property	Value	Unit
❶ Cover PP or PE nonwoven mass per unit area (MPUA)	300	g/m <sup>2</sup>
❷ Carrier PP or PE woven MPUA	110	g/m <sup>2</sup>
❸ Sodium bentonite powder MPUA	> 9,000	g/m <sup>2</sup>
❹ Sodium bentonite moisture content at delivery	approx. 10	%
GCL total MPUA	10,400	g/m <sup>2</sup>
GCL thickness	9	mm
GCL tensile strength (machine and cross-machine direction)	12	kN/m
GCL CBR (plunger puncture) strength	2,000	N
Permittivity	$< 5 \cdot 10^{-9}$	1/s
Permeability	$2 \cdot 10^{-11}$	m/s
Bentonite impregnated length overlaps for self-sealing	0.5	m
Roll weight	< 1	t

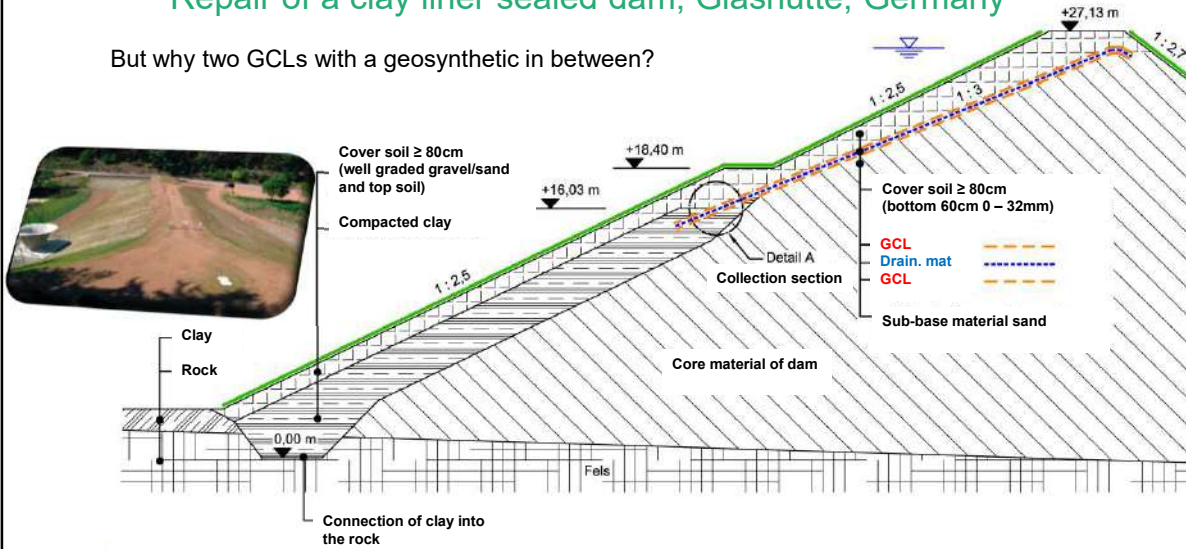


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Huge stormwater retention dam

Repair of a clay liner sealed dam, Glashütte, Germany

But why two GCLs with a geosynthetic in between?



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**Huge stormwater retention dam**

### Repair of a clay liner sealed dam, Glashütte, Germany

Detail A – Collection and control section

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**Huge stormwater retention dam**

### Repair of a clay liner sealed dam, Glashütte, Germany

Bird's view of construction site

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**Huge stormwater retention dam**

**Repair of a clay liner sealed dam, Glashütte, Germany**



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**Hohenwarte II pump storage station**

**Geosynthetic Clay Liner as Primary Lining System**

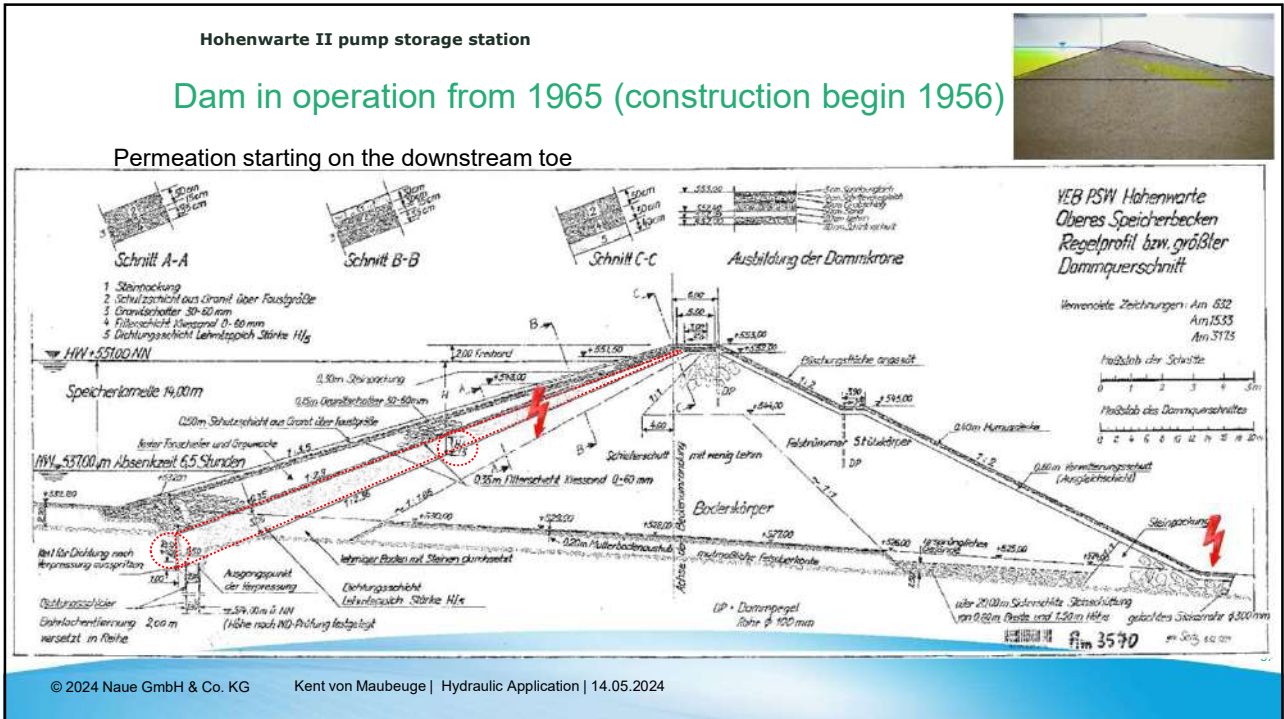
Permeation starting on the downstream toe



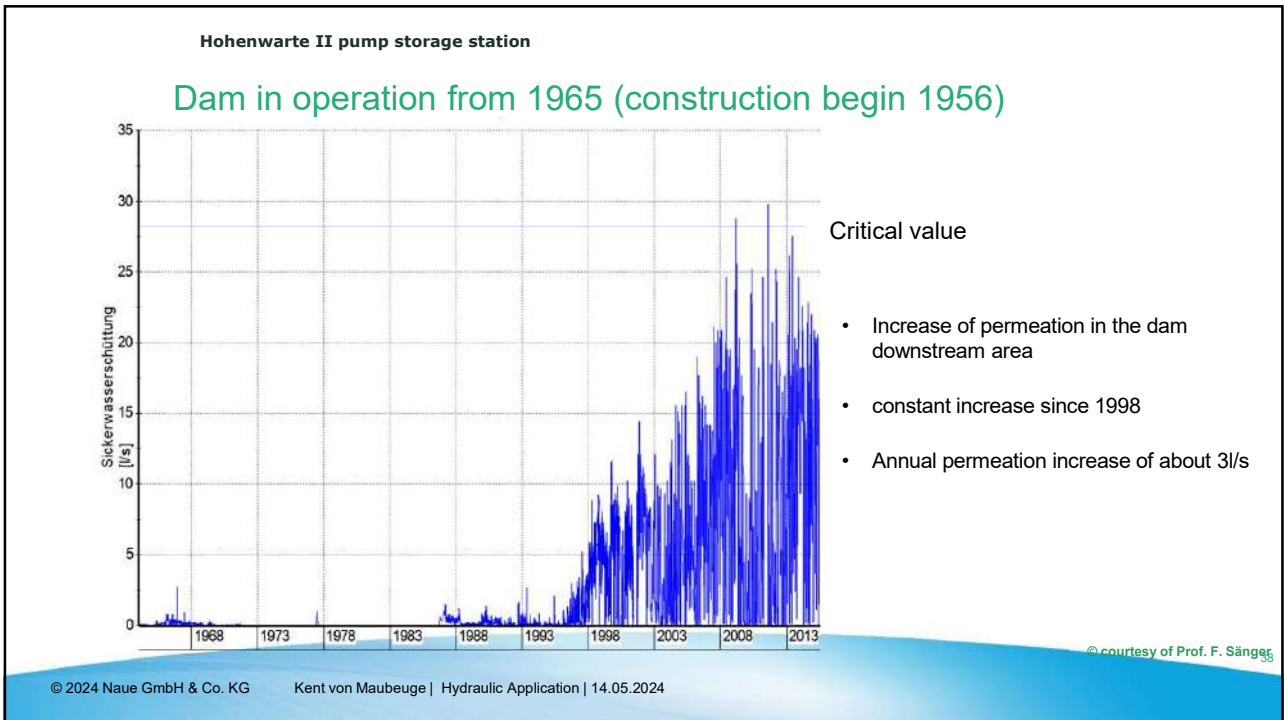
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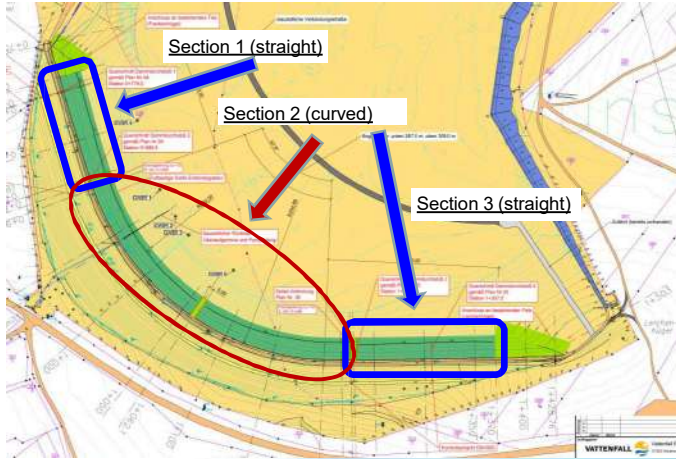


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Hohenwarte II pump storage station

### Selected possible repair solutions

Sections of repair



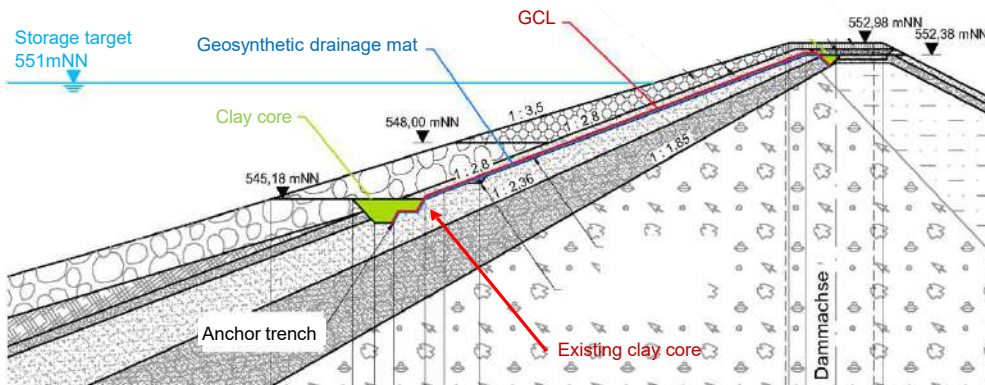
- Local repair was not an option
- New clay liner – time intense, costs
- Asphalt concrete – connections, technology
- Briposan – not enough experience
- Geomembrane – connections, stiffness
- Geosynthetic clay liner – cost efficient, positive experience in Glashütte, self-sealing potential

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Hohenwarte II pump storage station

### Proposed design solution

Sections of repair



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Hohenwarte II pump storage station

Specified GCL properties and requirements



Property	Value	Unit
❶ Cover PP or PE nonwoven mass per unit area (MPUA)	300	g/m <sup>2</sup>
❷ Carrier PP or PE woven MPUA	110	g/m <sup>2</sup>
❸ Sodium bentonite powder MPUA	> 9,000	g/m <sup>2</sup>
❹ Sodium bentonite moisture content at delivery	approx. 10	%
GCL total MPUA	10,300	g/m <sup>2</sup>
GCL thickness	12	mm
GCL tensile strength (machine and cross-machine direction)	12	kN/m
Permeability	< 1 · 10 <sup>-11</sup>	m/s
Bentonite impregnated length overlaps for self-sealing	0.5	m
Roll length	27	m
System friction angle (EC7, section 2.4.5)	26	°
Acceptable mean flow rate per unit area through the GCL	0.25	ml/s per m <sup>2</sup>

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Hohenwarte II pump storage station

Repair and GCL installation phase



GCL installation wit 0.5m overlaps  
Placement of geosynthetic drainage mat



GCL anchoring in the clay core

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Hohenwarte II pump storage station

Bird's view of construction site



Sequence of construction

© Prof. F. Sängler

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Hohenwarte II pump storage station

Entire view of construction site

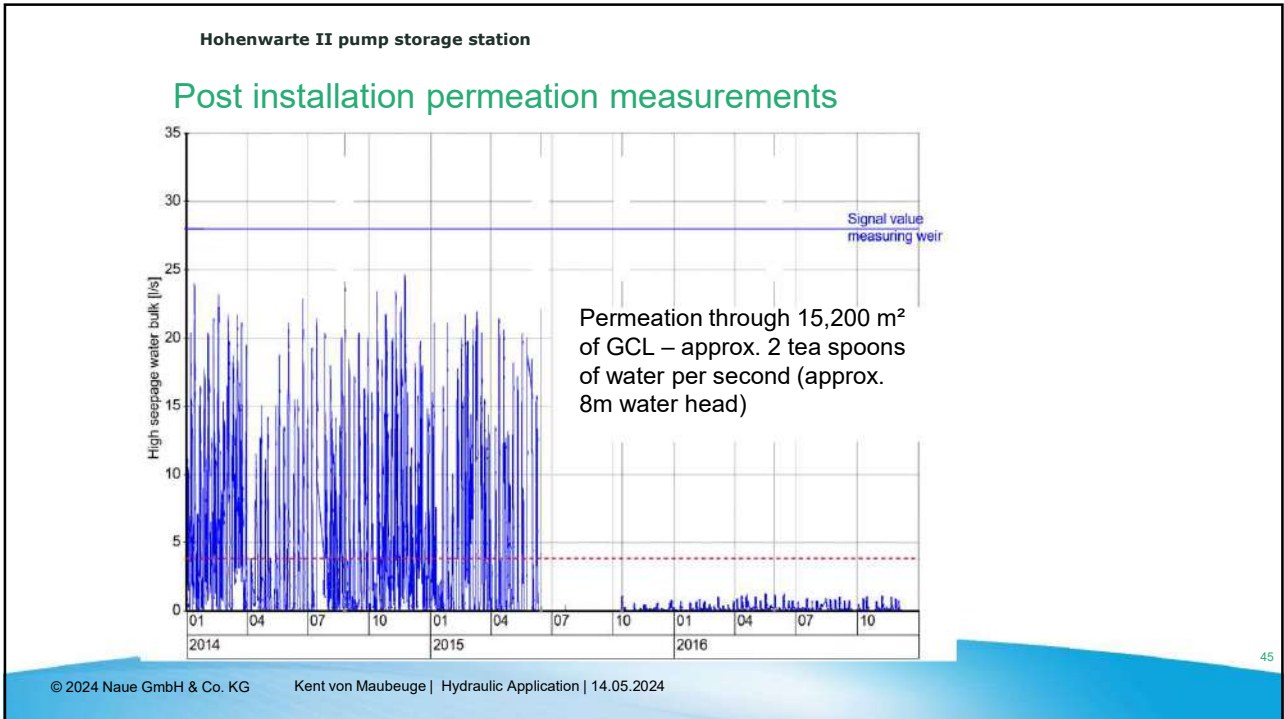


Bird's view

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**Geosynthetic clay liners**

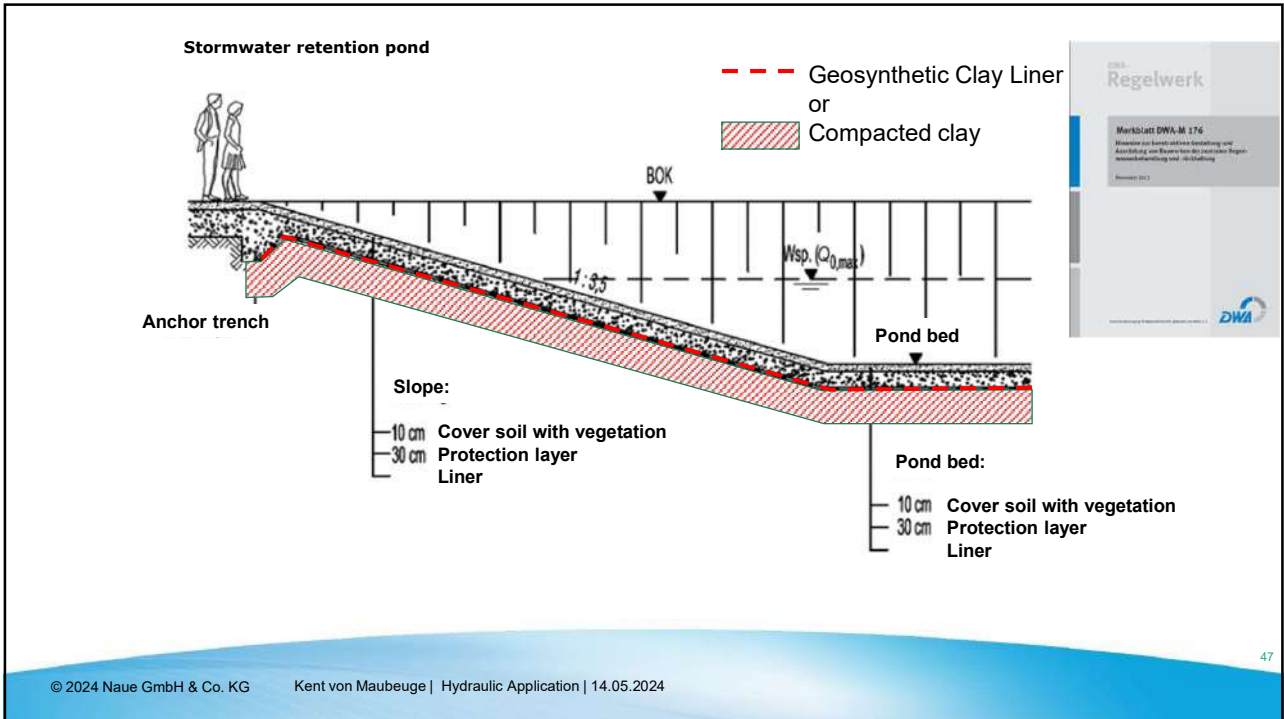
### Sustainable, ecological, economical, resilient

Less transportation vehicles  
Less noise emission  
Less impact to roads

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**Soil classification in waste groups and approx. Deposit pricing**

**Market prices for landfill storage**  
 [Bauindustrieverband Niedersachsen / Bremen, 2014]

		Ca. 1,75 to/cbm
April 2020 ca. 40 €/to ca. 50 €/to ca. 65 €/to	Boden bis LAGA Z0	5,00 – 6,00 Euro/cbm
	Boden bis LAGA Z1.1	9,00 – 11,00 Euro/cbm
	Boden bis LAGA Z1.2, DK0	19,00 – 22,00 Euro/to
	Boden bis LAGA Z2, DK0	22,00 – 26,00 Euro/to
	Boden bis LAGA > Z2	28,00 – 32,00 Euro/to
	Boden LAGA Z3, DK1	38,00 – 50,00 Euro/to
		5,00 – 6,00 Euro/cbm
		9,00 – 11,00 Euro/cbm
		33,25 – 38,50 Euro/cbm
		38,50 – 45,50 Euro/cbm
		49,00 – 56,00 Euro/cbm
		66,50 – 84,50 Euro/cbm

**For the following example 35 €/m<sup>3</sup> were estimated (likely now 75 – 80 €/m<sup>3</sup>)**

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**The more soil you excavate, the more might have to go to a landfill**

Excavation:  $3,000\text{m}^3 \cdot 0.5\text{m} \cdot 5\text{€/m}^3 = 7,500 \text{ €}$

Transportation of  $1,500\text{m}^3$  (approx. 110 trucks)  $\cdot 10 \text{ €/m}^3 \Rightarrow 15,000 \text{ €}$

Landfilling of  $1,500\text{m}^3 \cdot 35 \text{ €/m}^3 = 52,500 \text{ €}$   
 Total extra-costs with clay:  $75,000 \text{ €}$  for a  $3,000 \text{ m}^2$  pond

Storage till re-use of  $1,500\text{m}^3 \cdot 5\text{ €} = 7,500\text{ €}$   
 Total extra-costs with clay:  $30,000 \text{ €}$  for  $3,000 \text{ m}^2$  pond

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## GCL and sustainability

### How much water is needed for a compacted clay liner?

Area:  $40,000\text{m}^2$  - Clay thickness:  $0.5\text{m}$   
 Need to add approx.  $1,500,000 \text{ l}$  of water

Average Daily Water Usage Per Person

← America 580 l/day

Africa 5-10 l/day

What is the minimum quantity of water needed?  
 20 litres per person per day (205 person/year!)

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**Fact 17: Transportation benefits GCLs**

Example:

**4,500m<sup>2</sup> sealing with Bentofix® GCL**

**4,500m<sup>2</sup> sealing with compacted clay (500mm thick)**

Equals:

Equals:

**1 truck**

**187 trucks**



**Installation of a compacted clay liner**



1 Project size 4.500m<sup>2</sup>



2 187 truck loads



3 Stress on the access roads



4 Large storage area



5 in situ mixing



6 Transportation on site



7 1st clay layer



8 Watering



9 Compaction of 1st layer



10 Vibration compaction



11 2nd clay layer



12 Watering



13 Compaction 2nd layer



14 Vibration compaction



15 Desiccation



16 Cover soil placement



17 Cover soil movement

### Installation of a compacted clay liner

Project size 4.500m<sup>2</sup> → Only 1 truck load → Easy unloading → Small storage area →

→ Fast on site delivery → Easy installation → Self-sealing overlaps →

→ Cover soil placement → Cover soil distribution → Done

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### Comparable Results of Cumulated Energy Demand (CED)

#### Comparison of energy demand [MJ/m<sup>2</sup>] – 36,000 m<sup>2</sup>

**Barrier with GBR-C/GCL**

70.8 MJ/m<sup>2</sup> 😊

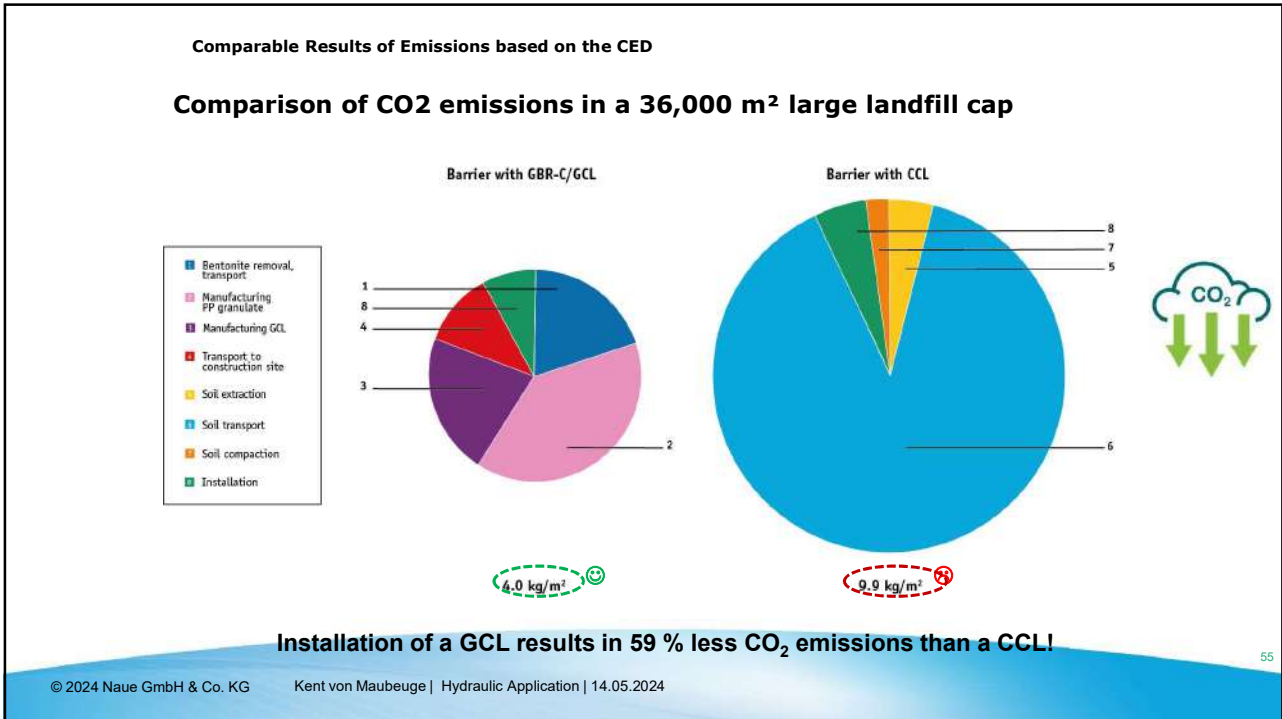
**Barrier with CCL**

122.3 MJ/m<sup>2</sup> 😞

**Installation of a GCL results in 42 % less energy demand than a CCL!**

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### Infographic – GCL vs CCL

14 Important facts why a Bentofix® geosynthetic clay liner (GCL) outperforms a compacted clay liner (CCL)

- 01** Typically regulations or recommendations suggest 0.5m thick clay liners as a sealing element (with a permeability of e.g.  $k \leq 5 \cdot 10^{-9}$  m/s). According to Darcy's law the permeation rate at a 0.3m water head would be 6.91 m<sup>3</sup>/ha/day. A Bentofix® GCL with  $k \leq 5 \cdot 10^{-11}$  m/s would have - under same conditions - a permeation rate of only 1.34 m<sup>3</sup>/ha/day.
- 02** Will P. Gates et al. (2009) calculated the time in years it takes for 10cm of fluid to permeate through the system:
 

	CCL (100cm)	GCL (1 cm thick)
Deionised water	9.5 years	41 years
0.1 mol NaCl	2 years	10 years
- 03** While the permeation rate of a compacted clay liner (0.5m thick,  $k \leq 1 \cdot 10^{-9}$  m/s, 0.3m water head) calculates theoretically to 1.38 m<sup>3</sup>/ha/day, a GCL (1cm thick,  $k \leq 5 \cdot 10^{-11}$  m/s, 0.3m water head) calculates theoretically to 1.34 m<sup>3</sup>/ha/day. However, field excavations after installation on a clay liner (Rogowski, 1986)) show much higher values, e.g. 12.07 m<sup>3</sup>/ha/day, which were confirmed by Daniels (1994).
- 04** Data from excavations from several landfills indicate that the installation performance (poor, good or excellent) influences the performance of clay liners. To achieve the regulated permeability value  $k = 1 \cdot 10^{-9}$  m/s the 0.5m thickness should be increased to approx. 0.8m to ensure the required regulated k value (assumption: good installation).
- 05** A perfectly installed clay liner (0.5m thick,  $k \leq 1 \cdot 10^{-9}$  m/s) in a landfill cap in a research project worked very well for the first 4 years, with low permeation rates but in the following 4 years the permeation rates were always in the range 50 - 200mm per year (Melchior, 2010). A Bentofix® GCL in a 20 year period was always lower than 7mm per year and in a 5 year average less than 20mm/year (Müller-Kirchenbauer, 2016).

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### Infographic – GCL vs CCL

14 Important facts why a Bentofix® geosynthetic clay liner (GCL) outperforms a compacted clay liner (CCL)

- 06** With enough confining stress (e.g. 1m cover soil) a CCL and a GCL can self-seal once desiccated if enough water makes it to the lining system. However, a 0.5m thick clay liner would need 21 liters of water, while a GCL only needs 1.5 liters of water per m<sup>2</sup>.
- 07** CCL has an average tensile strain at failure of 0.31%. Settlements in landfill caps can be much more. Heerten and Koerner (2008) have reported deformations varies from 1.8% to 27.4% in the CCL after seven years of monitoring of a 25-hectar landfill cap. This is far more than 0.31%. Needle-punched Bentofix® GCLs can have high tensile strain and withstand differential settlements (depending on product type 15% - 30%).
- 08** CCLs are placed and compacted in shifts on site, to achieve the required thickness. Due to the varying nature of clay and affecting parameters (e.g. compaction, moisture content, desiccation, etc.) it is difficult to achieve a uniform clay quality. Quality control on site requires to damage the CCL and the repair could cause leakage. GCLs are factory produced under controlled conditions and are installed without compaction or added water.
- 09** On earth we only have a limited amount of fresh water. Compacted clay liners need to be installed at an optimum moisture content and therefore need to be treated with fresh water on site. For a 40,000m<sup>2</sup> area approx. 2,340m<sup>3</sup> fresh water is needed. Unnecessary wasted fresh water. A Bentofix® GCL is installed in dry state and needs no moistening with fresh water, as the bentonite hydrates with the surrounding soil moisture or rain water.
- 10** Transportation with trucks has an influence on the long-term performance of roads and generates environmental and noise pollution. With one truck load approx. 4.500m<sup>2</sup> GCL can be delivered on site. To deliver the same amount of compacted clay in 0.5m thickness 187 truck loads of clay are necessary.

### Infographic – GCL vs CCL

14 Important facts why a Bentofix® geosynthetic clay liner (GCL) outperforms a compacted clay liner (CCL)

- 11** It is known that sodium bentonite can undergo an ionic exchange and that under certain conditions (e.g. low confining stress) the permeability of the GCL can slightly increase. However, with a higher bentonite mass per unit area and confining stress the increase of the permeability can be reduced.
- 12** Compacted clay liners are often 500mm thick. A geosynthetic clay liner (GCL) with approx. 10mm thickness offers economical and ecological advantages. It requires less excavation and therefore likely less material to deposit in a landfill, both saving enormous costs in excavation and transportation, with less impact for the environment. For a 3,000m<sup>2</sup> stormwater retention pond the savings where just on waste deposit approx. 28,500€.
- 13** Transportation of clay (0.5m thick) to the construction site (35km) and the installation for a landfill cap (including cover soil placement) considering the whole life cycle of the clay (mining and processing of raw materials, production, distribution and transport, usage, consumption und disposal) has a energy demand of 122.3 MJ/m<sup>2</sup>, while a GCL only consumes 70.8 MJ/m<sup>2</sup> (transportation 580km).
- 14** Transportation of clay (0.5m thick) to the construction site (35km) and the installation for a landfill cap(including cover soil placement) considering the whole life cycle of the clay (mining and processing of raw materials, production, distribution and transport, usage, consumption und disposal) has a CO<sub>2</sub> output of 9.9 kg/m<sup>2</sup>, while a GCL only generates 4.0 kg/m<sup>2</sup> (transportation 580km).
- 15** There are other project-dependent advantages in favor of the bentonite mat, such as diffusion, freeze-thaw cycles, replastication, self-sealing overlaps, etc.



Geosynthetic Clay Liners in hydraulic applications

**Thank you very much for attending and listening**

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